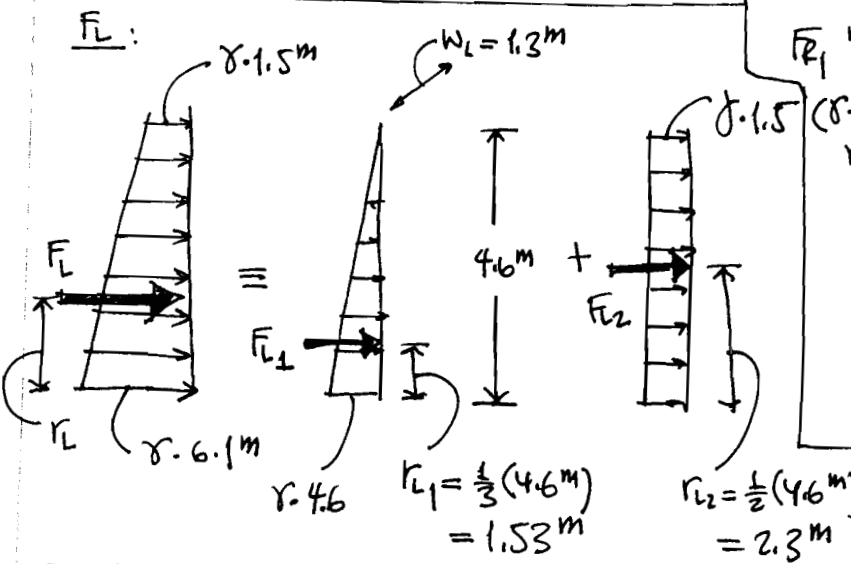
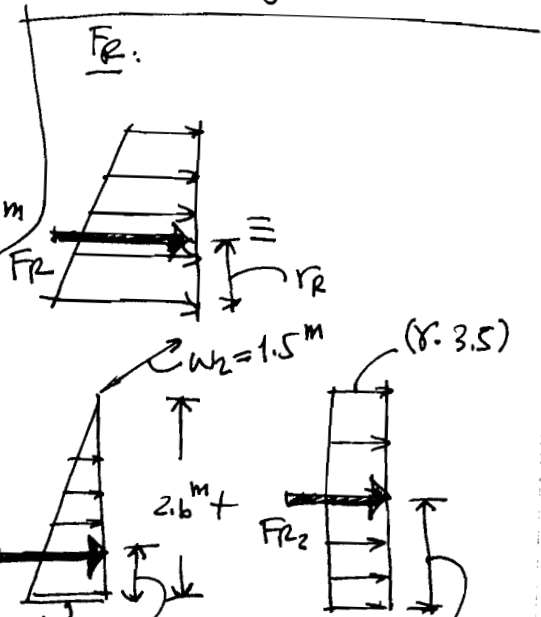
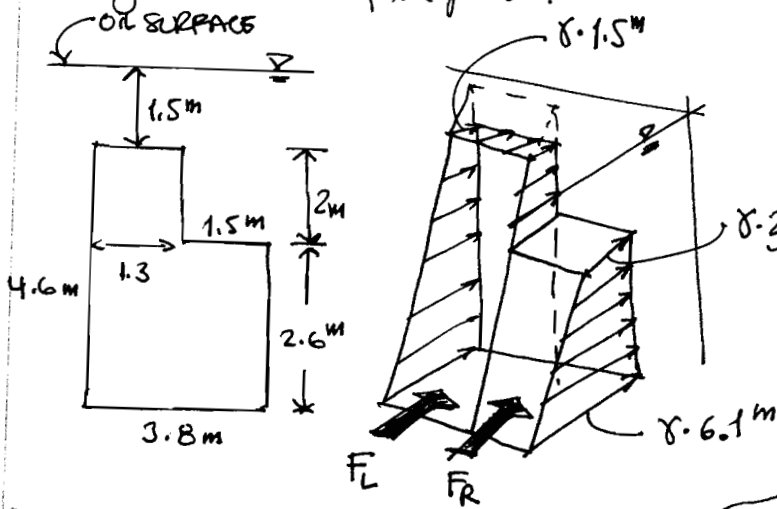


3.14. The Utah-shaped plate shown in Fig. P3.14 is submerged in oil ($s=0.94$) and lies in a vertical plane. Find the magnitude and locations of the hydrostatic force acting on one side of the plate.



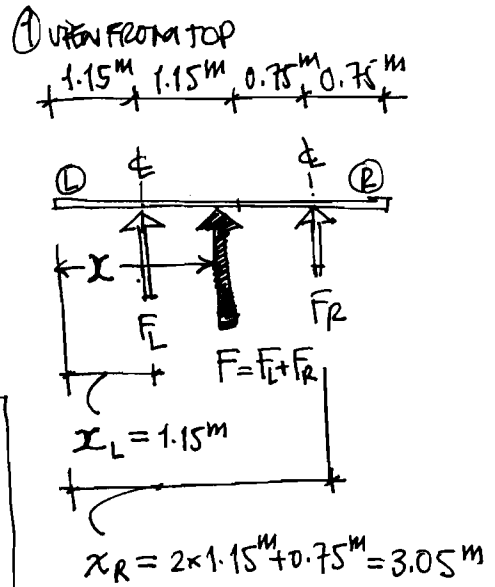
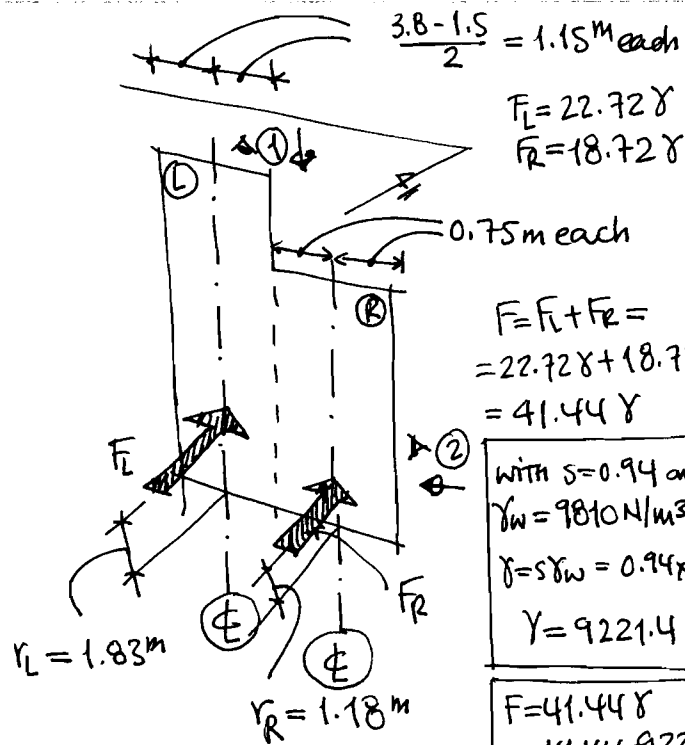
$r_{R1} = \frac{1}{3} \times (2.6) = 0.87\text{m}$
 $r_{R2} = \frac{1}{2} (2.6) = 1.3\text{m}$
 $F_{R1} = \frac{1}{2} (\gamma \times 2.6) \times (2.6) \times (1.5) = 5.07\gamma$
 $F_{R2} = (\gamma \times 3.5) (2.6) \times (1.5) = 13.65\gamma$
 $F_R = F_{R1} + F_{R2} = 5.07\gamma + 13.65\gamma = 18.72\gamma$

$F_{L1} = \frac{1}{2} (\gamma \times 4.6) (4.6) \times 1.3 = 13.754\gamma$
 $F_{L2} = (\gamma \times 1.5) (4.6) \times 1.3 = 8.97\gamma$
 $F_L = F_{L1} + F_{L2} = 13.754\gamma + 8.97\gamma = 22.724\gamma$

C.P.L: $r_L \cdot F_L = r_{L1} \cdot F_{L1} + r_{L2} \cdot F_{L2}$
 $r_L \cdot 22.724\gamma = 1.53 \times 13.754\gamma + 2.3 \times 8.97\gamma$
 $r_L = \frac{41.675}{22.724} = 1.83\text{m}$

C.P.R: $r_R \cdot F_R = r_{R1} \cdot F_{R1} + r_{R2} \cdot F_{R2}$
 $r_R \cdot 18.72\gamma = 0.87 \times 5.07\gamma + 1.3 \times 13.65\gamma$
 $r_R = \frac{22.156}{18.72} = 1.18\text{m}$

continues in next page



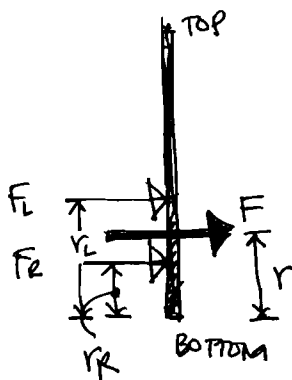
To find x for total force

$$x \cdot F = x_L \cdot F_L + x_R \cdot F_R$$

$$x \cdot 41.44 \gamma = 1.15 \times 22.72 \gamma + 3.05 \times 18.72 \gamma$$

$$x = \frac{91.648}{41.44} = 2.21 \text{ m}$$

② VIEW FROM SIDE

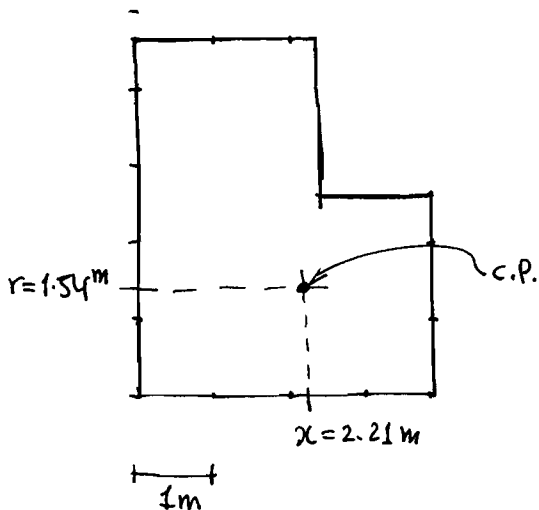


⇒ TO FIND r for the total force

$$r \cdot F = r_L \cdot F_L + r_R \cdot F_R$$

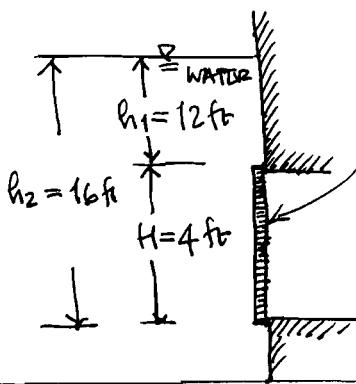
$$r \cdot 41.44 \gamma = 1.83 \times 22.72 \gamma + 1.18 \times 18.72 \gamma$$

$$r = \frac{63.67}{41.44} = 1.54 \text{ m}$$

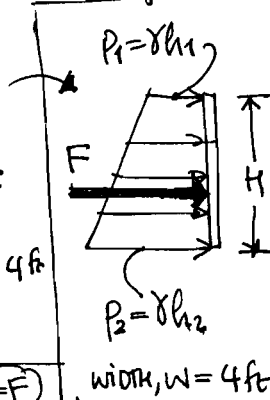


3.15. The common type of irrigation head gate shown in Fig. P3.15 is a plate that slides over the opening to a culvert. The coefficient of friction between the gate and its sliding ways is 0.6. Find the force required to slide open this 600-lb gate if it is set (a) vertically; (b) on a 2:1 slope ($n=2$) as is common.

(a) set vertically



48 in square gate
 $48 \text{ in} = \frac{48}{12} = 4 \text{ ft}$



$$F = \text{volume of pressure prism} = \frac{1}{2}(P_1 + P_2) \cdot H \cdot W$$

$$= \frac{1}{2} \gamma \left(\frac{h_1 + h_2}{2} \right) H \cdot W$$

HYDROSTATIC FORCE CALCULATION

$$\gamma = 62.4 \text{ lb/ft}^3$$

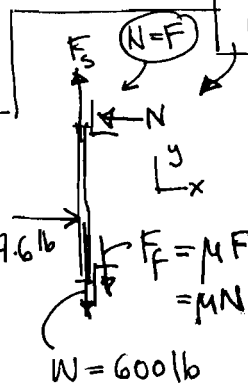
$$F = (62.4 \frac{\text{lb}}{\text{ft}^3}) \left(\frac{12 \text{ ft} + 16 \text{ ft}}{2} \right) \cdot 4 \text{ ft} \cdot 4 \text{ ft}$$

$$F = 13977.6 \text{ lb}$$

Analysis of forces including friction:

normal force, $F = 13977.6 \text{ lb}$

coefficient of friction, $\mu = 0.6$



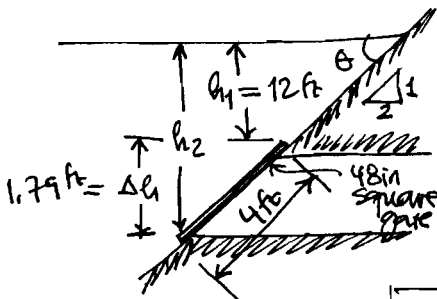
F_s = minimum force required to slide open (upwards)

$$\sum F_y = 0 \text{ (zero acceleration, sliding at constant velocity)}$$

$$F_s - W - \mu F = 0 \Rightarrow F_s = W + \mu F$$

$$F_s = 600 \text{ lb} + 0.6 \times 13977 \text{ lb} = 8986.56$$

(b) set a slope 2:1

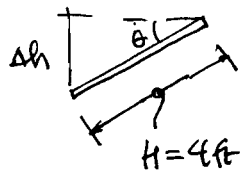


$$\frac{\text{opposite}}{\text{adjacent}} = \tan \theta = 1/2 \Rightarrow \theta = \tan^{-1}(1/2) = 26.57^\circ$$

GATE:

$$\sin \theta = \frac{\Delta h}{H} \Rightarrow \Delta h = H \sin \theta = 4 \text{ ft} \sin 26.57^\circ$$

$$\Delta h = 1.79 \text{ ft}$$

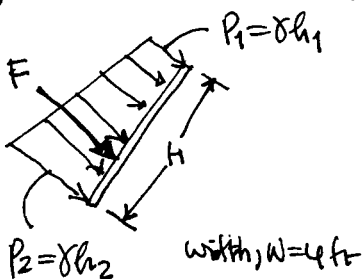


$$h_2 = h_1 + \Delta h = 12 \text{ ft} + 1.79 \text{ ft} = 13.79 \text{ ft}$$

hydrostatic force calculation

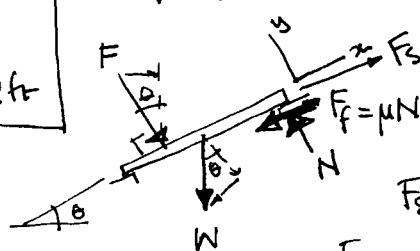
$$F = \text{volume of pressure prism} = \gamma \left(\frac{h_1 + h_2}{2} \right) H \cdot W = (62.4 \frac{\text{lb}}{\text{ft}^3}) \left(\frac{12 \text{ ft} + 13.79 \text{ ft}}{2} \right) (4 \text{ ft})(4 \text{ ft})$$

$$F = 12874.37 \text{ lb}$$



coefficient of friction, $\mu = 0.60$

Analysis of forces



$$\sum F_y = 0, -F - W \cos \theta + N = 0$$

$$N = F + W \cos \theta$$

$$= 12874.37 + 600 \cos 26.57^\circ$$

$$= 13411.00 \text{ lb}$$

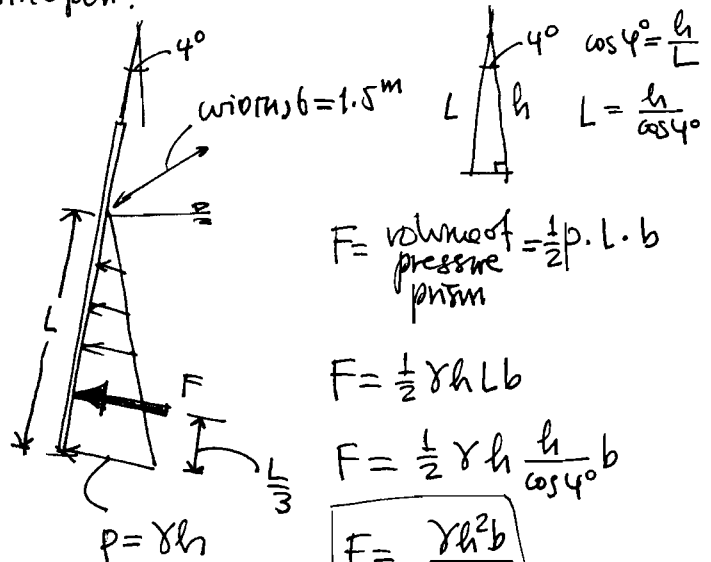
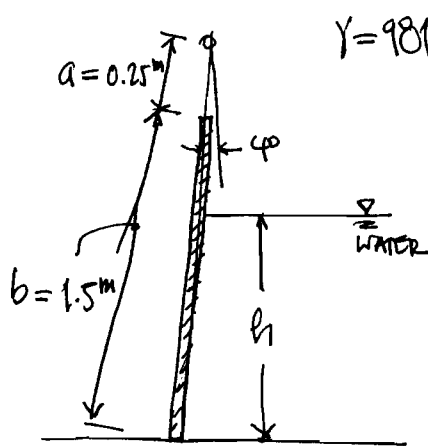
$$\sum F_x = 0, F_s - F_f - W \sin \theta = 0$$

$$F_s = F_f + W \sin \theta = \mu N + W \sin \theta$$

$$F_s = 0.6 \times 13411.00 \text{ lb} + 600 \sin 26.57^\circ$$

$$F_s = 8314.97 \text{ lb} \approx 8315 \text{ lb}$$

3.16. In the drainage of irrigated lands it is frequently desirable to install automatic flap gates to prevent a flood from backing up into the lateral drains from a river. Suppose a square flap gate, of side $b = 1.5\text{ m}$ and weight 8 kN , is hinged 1 m above its center ($a = 0.25\text{ m}$), as shown in the figure, and the face is sloped 4° from the vertical. To what depth will water rise behind the gate before it will open?



$$F = \text{volume of pressure prism} = \frac{1}{2} p \cdot L \cdot b$$

$$F = \frac{1}{2} \gamma h L b$$

$$F = \frac{1}{2} \gamma h \frac{h}{\cos 4^\circ} b$$

$$F = \frac{\gamma h^2 b}{2 \cos 4^\circ}$$

$$\frac{L}{3} = \frac{h}{3 \cos 4^\circ}$$

FORCES THAT PRODUCE A MOMENT ABOUT O:

FORCE	ARM
$W \sin 4^\circ$	$a + b/2$
$F = \frac{\gamma h^2 b}{2 \cos 4^\circ}$	$a + b - \frac{h}{3 \cos 4^\circ}$

The minimum h required to make the gate rise is such that $\sum M_o = 0$

$$-W \sin 4^\circ \times \left(a + \frac{b}{2}\right) + \frac{\gamma h^2 b}{2 \cos 4^\circ} \times \left(a + b - \frac{h}{3 \cos 4^\circ}\right) = 0$$

\Rightarrow ~~quadratic~~ cubic equation: $-\frac{\gamma b}{6 \cos^2 4^\circ} h^3 + \frac{(a+b) \gamma b}{2 \cos 4^\circ} h^2 - W \sin 4^\circ \left(a + \frac{b}{2}\right) = 0$

$$\alpha_1 = \frac{\gamma b}{6 \cos^2 4^\circ} = \frac{9810 \times 1.5}{6 \cos^2 4^\circ} = 2464.99$$

$$\alpha_2 = \frac{(a+b) \gamma b}{2 \cos 4^\circ} = \frac{(0.25 + 1.5) \times 9810 \times 1.5}{2 \cos 4^\circ} = 12907.07$$

$$\alpha_3 = W \sin 4^\circ \left(a + \frac{b}{2}\right) = 8000 \times \sin 4^\circ \times \left(0.25 + \frac{1.5}{2}\right) = 558.05$$

$$-\alpha_1 h^3 + \alpha_2 h^2 - \alpha_3 = 0$$

$$-2464.99 h^3 + 12907.07 h^2 - 558.05 = 0$$

solutions: $h = 0.21\text{ m}$
 $h = 5.22\text{ m}$
 $h = -0.20\text{ m}$